ABSTRACT: Nankai Earthquake has occurred at the interval of 90-150 years. The probability is said to be M8.4, with 50% within 30 years and with 80% within 50 years. There is a big possibility of liquefaction because of the loose sand at the coastal zone of the Kochi Prefecture. It is required to investigate the liquefaction potential as accurate as possible for the evacuation plan. To accurately examine the evaluation of liquefaction potential, the cyclic triaxial test by undisturbed samples were executed. This research was executed to the Kaminokae Fishing Village, where is the representative model district, and to other places. The simulated earthquake at the bedrock, which was made on the assumption of magnitude 8.4 and the hypocenter on the Nankai Trough was used.

The prediction of liquefaction was performed based on “Guidance of Facilities Design of Fishing Village and Fishery (version 2003)”, “Simplified Prediction Method (SPM) of Specification for Highway Bridges(version 2002) ” and “Momentary Deformation Modulus Method (MDM)”. The conclusions of this study are: (1) Guidance of Facilities Design of Fishing Village and Fishery gives strong liquefaction potential, (2) SPM gives the stronger liquefaction potential compared with MDM, and (3) Simplified Prediction Method of Specification for Highway Bridges is recommendable as the Basic guideline of liquefaction evaluation method for Fishing Village and Fishery.

KEYWORDS: Liquefaction, Nankai Earthquake, MDM method

1. INTRODUCTION

The 130 fishing ports are scattered at the coast of Kochi Prefecture, where 69,000 people in the 109 villages live in this area, and will be expected to be suffered from the serious damage of the earthquake motion and the tsunami by Nankai Earthquake. The purpose of this study is to predict the ground liquefaction potential by Nankai Earthquake, as a part of the plan of basic policy for evacuation. This paper focuses the prediction method of grand liquefaction. The liquefaction prediction by the three detailed methods was performed for the Kaminokae district in Nakatosa-cho to compare these three methods. Then, the prediction method of the ground liquefaction in the fishing village was proposed. Moreover, the hazard map of the ground liquefaction in this area was made.

2. PREDICTION METHOD OF GROUND LIQUEFACTION

The characteristics of Hanshin-Awaji Earthquake on
the ground liquefaction which occurred on January 17, 1995, are as follows to include these items to liquefaction prediction method:

1) The liquefaction was generated in the large shore reclaimed area, where was composed of decomposed granite including a wide-range particle size with gravel.

2) Liquefaction was generated at the soil of the rate of fine-grained particle 35% or less.

3) The lateral flow occurred in the large area with the generation of liquefaction.

“Guidance of Facilities Design of Fishing Village and Fishery (version 2003)” and “Simplified Prediction Method (SPM) of Specification for Highway Bridges (version 2003)” were revised based on the abovementioned characteristics.

The points of the liquefaction prediction in Kaminokae district are shown in Fig.1. For the detailed estimation of liquefaction, the four borings (Nos. A-D) to reach to the bedrock were performed in addition to existing 23 borings to confirm the soil strata to do the necessary soil test for the liquefaction prediction. The geological section along the coast line was made based on the result of the survey, as shown in Fig.2. The strata consist of six layers of which the soil test results are shown in Table 1. The geologic column and the distribution of N value of point A are shown in Fig.3. In addition, the frozen undisturbed samples were taken in the layers at point A to perform the cyclic triaxial compressin test and to examin the possibility of lliquefaction. The samplings were done in the following depths:

1). 8.5-10.5m in the second layer (sand)
2). 10.8-17m in the third layer (silt)
3). 21.8-24.0m in the fifth layer (silt)

The liquefaction prediction methods were as follows:

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### Table 1  Result of Soil Test

<table>
<thead>
<tr>
<th>Stratum Name</th>
<th>Result of Soil Test</th>
<th>FC %</th>
<th>D10 mm</th>
<th>D50 mm</th>
<th>Ip</th>
</tr>
</thead>
<tbody>
<tr>
<td>Sandy gravel with Silt</td>
<td></td>
<td>13.475</td>
<td>0.0238</td>
<td>4.2969</td>
<td>9.3</td>
</tr>
<tr>
<td>Sand</td>
<td></td>
<td>28.8</td>
<td>0.046</td>
<td>0.129</td>
<td>-</td>
</tr>
<tr>
<td>Silt</td>
<td></td>
<td>84.4</td>
<td>0.001</td>
<td>0.0137</td>
<td>25.15</td>
</tr>
<tr>
<td>Volcanic ash</td>
<td></td>
<td>38.7</td>
<td>0.0042</td>
<td>0.1096</td>
<td>-</td>
</tr>
<tr>
<td>Silt</td>
<td></td>
<td>96.2</td>
<td>0.001</td>
<td>0.0049</td>
<td>28.25</td>
</tr>
<tr>
<td>Sandy gravel with Silt</td>
<td></td>
<td>22.4</td>
<td>0.0078</td>
<td>1.4828</td>
<td>8.35</td>
</tr>
</tbody>
</table>

FC: Rate of fine-grained soil  D10: Effective grain size  D50: Average grain diameter  Ip: Plasticity index

According to Guidance of Facility Design of Fishing Port and Fishery, the prediction flow of liquefaction is shown in Fig.4. The prediction is performed by the following three steps:

1) **Evaluation by grain size**
   The liquefaction potential of each layer is evaluated by the uniformity coefficient and the coefficient of permeability. The borderline of the uniformity coefficient is 3.5.

2) **Prediction and evaluation by equivalent acceleration and equivalent N value**
   The category of liquefaction potential  -  , as shown in Table 2, is evaluated by the equivalent acceleration and equivalent N value of the each soil stratum.

   The equivalent acceleration should be obtained by the seismic response analysis such as the equivalent linearization method. In this study, MDM method was used as described later. The evaluation result by Method A is shown in Fig.5, and the evaluation method is shown in Table 2.

   **Table 2 Category of liquefaction by gradation and N value**

<table>
<thead>
<tr>
<th>Category</th>
<th>Prediction of liquefaction by gradation and N value</th>
<th>Prediction of liquefaction by gradation and N value</th>
</tr>
</thead>
<tbody>
<tr>
<td>□</td>
<td>Liquefies</td>
<td>It is estimated that the liquefaction occurs.</td>
</tr>
<tr>
<td>□</td>
<td>Strong possibility of liquefication</td>
<td>It is estimated that the liquefaction occurs, or it must be predicted by the cyclic triaxial test.</td>
</tr>
<tr>
<td>□</td>
<td>Bare possibility of liquefaction</td>
<td>It is estimated that the liquefaction does not occur, or it must be predicted by the cyclic triaxial test.</td>
</tr>
<tr>
<td>□</td>
<td>No possibility</td>
<td>It is estimated that the liquefaction does not occur.</td>
</tr>
</tbody>
</table>

3) **Guidance of Facility Design of Fishing Port and Fishery (Method A)**

![Geologic column](image)

**Fig.3** Boring log (Point A)

**Fig.4** Evaluation flow of liquefaction

**2.1 Guidance of Facility Design of Fishing Port and Fishery (Method A)**

![Evaluation](image)

**Fig.5** Liquefaction category at Point A
From the evaluation result of liquefaction as shown in Fig. 5, the liquefaction categories are □ and □ in the middle soil layers at the depth of 8 - 23 m. Since there is the possibility of liquefaction, the cyclic triaxial test was performed. The test results will be shown later.

3) Liquefaction evaluation by cyclic triaxial test
When the liquefaction evaluation was unable to evaluated by the liquefaction evaluation result 1), 2) results, the cyclic triaxial test of the undisturbed sampling was executed. And the shearing stress during earthquake was compared with the liquefaction resistance of ground. One example of the result of cyclic triaxial test is shown in Fig. 6 and 7. By the procedure of Method A, R = 0.36 was obtained from Fig. 7. Since L is 0.26 by Step2, FL value 1.39 was obtained.

2.2 Simplified Prediction Method (Method B)

1) Evaluation of liquefaction
The rate of liquefaction resistance FL is calculated by Formula (1) on the assumption of Type □ earthquake motion to the soil layers, which need the evaluation of liquefaction. The soil layer, which FL value is 1.0 or less, has the possibility of liquefaction.

\[ F_L = \frac{R}{L} \]  

where:
\( F_L \): rate of liquefaction resistance
\( R \): dynamic shearing strength ratio
\( L \): shearing stress ratio during earthquake

R is obtained by the relation between equivalent N value and the cyclic triaxial test results of the sandy soil, which is corrected by the rate of fine-grained soil, the relation between equivalent N value and the cyclic triaxial strength tests of the gravelly soil, and the characteristics by the types of earthquake motion (Type □ and Type □).

L is obtained by the Formula (2).

\[ L = \gamma_d K_{hfg} \sigma_v / \sigma_{v'} \]  

where:
\( \gamma_d \): decrease coefficient in the direction of depth
\( K_{hfg} \): standard design seismic coefficient of Level 2 earthquake motion
\( \sigma_v \): total overburden pressure
\( \sigma_{v'} \): effective overburden pressure

2) Liquefaction potential PL
PL value is obtained by Formula (3) from the
underground water level to GL-20m in depth.

\[ PL = \int_0^{20} (1 - F_{L,Z})(10 - 0.5z)dz \]  \hspace{1cm} (3)

where:

\( F_{L,Z} \): rate of liquefaction resistance in depth \( z \) \( (F_{L,Z} = 1 \text{ for } z \leq 1) \)

\( z \): depth(m)

2.3 Momentary Deformation Modulus Method (Method C)

Neither the amplification nor the damping of the earthquake wave by the soil structure was considered in Method B. The liquefaction evaluation was made by using Method C from the viewpoint of the accuracy for the liquefaction evaluation. The liquefaction of each stratum was evaluated by the rate of liquefaction resistance \( F_L \), and the liquefaction potential in plane was evaluated by \( P_L \) value.

□ Calculation of shearing stress ratio \( L \) during earthquake

Fig.8 shows the input seismic wave used for the seismic response analysis. This seismic wave is a wave profile on the engineering bedrock (N value : 50 or more) at Kaminokae district based on □The second basic study of earthquake in Kochi Prefecture □. The seismic response analysis was performed by the one-dimensional total stress seismic response analysis by using Method C. In MDM model, the strain dependency on the shearing rigidity \( G \) and the damping coefficient \( h \) as the nonlinear model parameter of each stratum are considered to a higher strain level. The soil parameters on the dynamic deformation characteristics for each stratum were adopted from the existing basic data of Public Works Research Institute and Ports and Harbors Research Institute. The energy dissipation was considered for the engineering bedrock which was half space ground and consecutive nonlinear analysis is performed in this analysis.

2) Calculation of cyclic shearing strength ratio \( R \)

In the calculation of the shearing strength ratio, the results of cyclic triaxial test conducted by the laboratory liquefaction test and the conventional method for liquefaction strength (Method B) were used. The example of the liquefaction evaluation result by method C at point A is shown in Fig.9.

3. DISCUSSION OF ANALYTICAL RESULTS

The example of the comparison on the liquefaction evaluation results by each method is shown in Fig.9. Although the layers except gravel layers have the poor possibility of liquefaction ( □ ), and evaluated not liquefaction from the cyclic triaxial test by Method A. Method B shows the occurrence of liquefaction without gravel layer and shows the conservative distribution of \( F_L \) value compared with Method C (\( P_L = 18 \) by Method C, \( P_L = 24 \) by Method B). In Method C, there is no meaningful difference
between the case of using the cyclic triaxial test data and the case of no use of the cyclic triaxial test data. From these results, Method B was proposed as the Basic Guidelines of liquefaction Evaluation method in the fishing village because of the conservativeness and the convenience to be applied for the liquefaction evaluation of coastal regions tentatively.

4. MAKING OF HAZARD MAP
The liquefaction hazard map in this area was made as shown in Fig.10 by $P_L$ value previously described. In this area, the strong possibility of liquefaction is observed in most area. It is understood that the liquefaction potential becomes small with leaving from the coastline.

5. CONCLUSION
The following conclusion were obtained as follows:
(1) The guidance of the facilities design of Fishing Village and Fishery gives the strong liquefaction potential.
(2) The simplified prediction method of Specification for Highway Bridges shows the same liquefaction tendency as the method of Momentary Deformation Modulus.
(3) The simplified prediction method of Specification for Highway Bridges is recommendable as the Basic guideline of liquefaction evaluation method for Fishing Village and Fishery.

And the future problems were clarified as follows:
1) It is necessary to confirm the reliability of the strong seismic motion to be input.
2) It is necessary to investigate the inference by the soil characteristics to analysis results.
3) It is thought that it will be necessary to make the hazard map in the fishing villages, and to apply it to "Tsunami evacuation plan" and the efficient effective hardware preparation plan in the future.

REFERENCES


